

1

- (a) - If $e < \delta_v$, the flow is in hydraulically smooth flow regime ^{shear}
 - If $e > 14 \delta_v$, the flow is in fully rough flow regime ^{viscous sublayer is lower than roughness}
 - If $\delta_v < e < 14 \delta_v$, the flow is in transitional rough flow regime ^{viscosity is not imp}

- (b) - For $Re \leq 1$, the flow is very viscous. The flow follows Stokes' Law: $F_D = 3\pi\mu VD$ or $C_D = 24/Re$
 - For $Re > 1$, laminar BL starts to separate from the surface of the sphere. As Re increases more, C_D starts to level off because now pressure drag is becoming more important and drag is proportional to V^2 . \neq When Re is from 10^3 to 10^4 , $C_D \approx 0.4$
 - At $Re = 2 \times 10^5$, C_D suddenly reduced by 50%.

(c) Manning's eq: $V = \frac{1}{n} R_h^{2/3} S_0^{1/2} \Rightarrow n = \frac{R_h^{2/3} S_0^{1/2}}{V}$ (1)

$\tau_0 = \frac{\rho f V^2}{8} \Rightarrow V = \sqrt{\frac{8\tau_0}{\rho f}}$
 $\Rightarrow n = \frac{R_h^{2/3} S_0^{1/2}}{\sqrt{\frac{8\tau_0}{\rho f}}} = \frac{R_h^{2/3} S_0^{1/2}}{\sqrt{\frac{\rho f}{8\tau_0}}}$

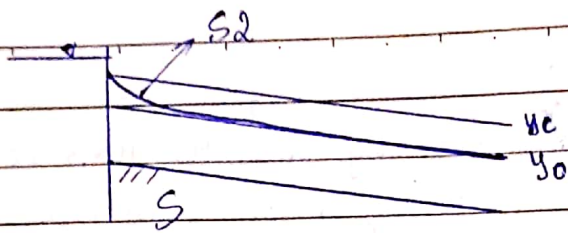
~~$\tau_0 = \frac{\rho f V^2}{8}$~~ $S_0 \approx S = \frac{hf}{L} = \frac{fV^2}{2gD} = \frac{fV^2}{2g(4R_h)} = \frac{fV^2}{8gR_h}$ (2)

Subst (2) into (1): $n = \frac{1}{V} \times R_h^{2/3} \sqrt{\frac{fV^2}{8gR_h}}$

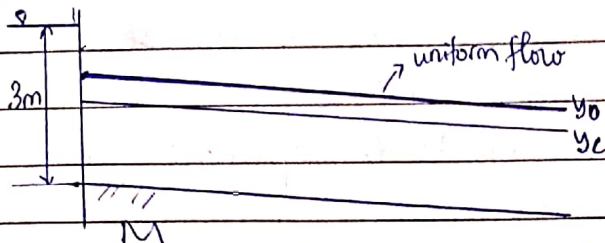
$\Rightarrow n = R_h^{1/6} \sqrt{\frac{f}{8g}}$

(d)

- (i) Flow from reservoir into steep slope \rightarrow critical condition at entrance. Since no energy is lost, $E_0 = E_c \Rightarrow E_c = 3m \Rightarrow y_c = \frac{2}{3} E_c = 2m$
 $\Rightarrow q = \sqrt{g y_c^3} = \sqrt{9.81 \times 2^3} = 8.86 \text{ m}^2/\text{s}$



(ii)

Entrance flow depth - $y_0 = 2.3\text{m}$ No energy loss $\Rightarrow E_0 = E_1$

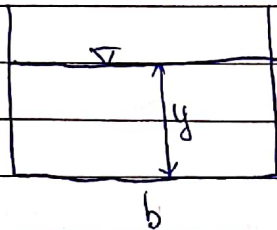
$$\Rightarrow 3 = y_0 + \frac{q^2}{2gy_0^2} = 2.3 + \frac{q^2}{2(9.81)(2.3^2)}$$

$$\Rightarrow q = 8.52 \text{ m}^2/\text{s}$$

2

- (a) - Boundary conditions: the curve has 2 asymptotes which are E-axis and line $y = F$.
- Between these two limits, there is a minimum E_c or E_{\min} . Any $E < E_c$ is physically impossible.
 - E_c is associated with a critical flow depth y_c .
 - For any E larger than E_c , there are two alternative depths, one larger than y_c (subcritical flow regime) and one smaller than y_c (supercritical flow regime).

(b)

Keep the cross-sectional area, A , as constant

$$A = by$$

$$P = b + 2y$$

$$R_h = \frac{A}{P} = \frac{by}{b + 2y}$$

$$Q = \frac{1}{n} A R_h^{2/3} S_0^{1/2} = \frac{1}{n} A \left(\frac{A}{P} \right)^{2/3} S_0^{1/2} = \frac{1}{n} \frac{A^{5/3}}{P^{2/3}} S_0^{1/2}$$

To maximize Q , we have to minimize P .

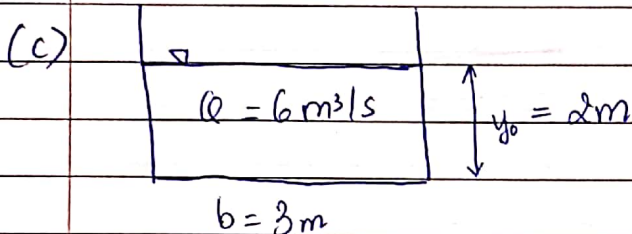
Since $A = by \Rightarrow b = \frac{A}{y}$

Substitute into P : $P = \frac{A}{y} + 2y$

Differentiate P with respect to y : $\frac{dP}{dy} = \frac{-A}{y^2} + 2$

Equate $\frac{dP}{dy}$ with 0 to find P_{min} : $\frac{-A}{y^2} + 2 = 0$

$\Rightarrow y = \sqrt{\frac{A}{2}}$ Also, $A = by$ } $\Rightarrow b = 2y$



(i) $q = Q/b = 2 \text{ m}^2/\text{s}$

$y_c = \sqrt[3]{\frac{q^2}{g}} = \sqrt[3]{\frac{2^2}{9.81}} = 0.742 \text{ m}$

Since $y_c < y_0$, the bed slope is Mild

(ii) $E_c = \frac{3}{2} y_c = \frac{3}{2} \times 0.742 = 1.113 \text{ m}$

(iii) Assume flow depth at section 1 $y_1 = y_0 = 2 \text{ m}$

$E_1 = y_1 + \frac{q^2}{2gy_1^2} = 2 + \frac{2^2}{2 \times 9.81 \times 2^2} = 2.051 \text{ m}$

At section 2, $E_2 = E_1 - \Delta z = 2.051 - 1.5 = 0.551 \text{ m}$

Since $E_2 < E_c$, choking occurs.

$\Rightarrow E_2 = E_c = 1.113 \text{ m}$ and $y_2 = y_c = 0.742 \text{ m}$

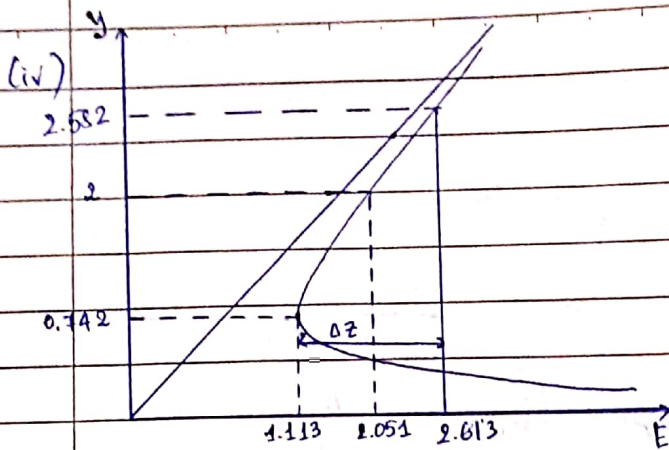
$E_1^* = E_2 + \Delta z = 1.113 + 1.5 = 2.613 \text{ m}$

$\Rightarrow y_1^* + \frac{2^2}{2(9.81)(y_1^*)^2} = 2.613 \text{ m}$

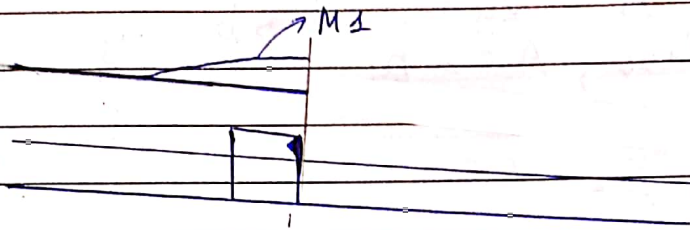
$\Rightarrow y_1^* = 2.582 \text{ m}$

\therefore Flow depth at section 1 = 2.582 m

Flow depth at section 2 = 0.742 m



(v)



3.

- (a) - Direct Step method assigns values to y to calculate x
 - Standard step method assigns values to x to calculate y
 - We must use standard step method when the channel is not prismatic

(b) $Q = 50 \text{ m}^3/\text{s}$, $n = 0.025$, $S_o = 3.75 \times 10^{-2}$, $b = 3 \text{ m}$

(i) $q = \frac{Q}{b} = \frac{50}{3} = 16.67 \text{ m}^2/\text{s}$

$$y_c = \sqrt[3]{\frac{q^2}{g}} = \sqrt[3]{\frac{16.67^2}{9.81}} = 3.048 \text{ m}, \text{ for both reaches}$$

- On the horizontal slope, normal flow depth does not exist

- For reach 1:

$$A = by_n = 3y_n$$

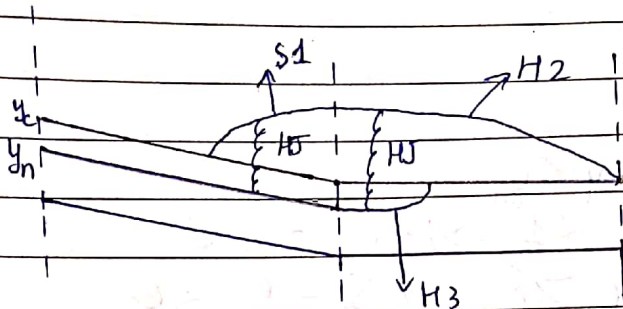
$$P = b + 2y_n = 3 + 2y_n$$

$$R = \frac{A}{P} = \frac{3y_n}{3 + 2y_n}$$

$$Q = \frac{1}{n} A R^{2/3} S_o^{1/2} \Rightarrow 50 = \frac{1}{0.025} \times (3y_n) \left(\frac{3y_n}{3 + 2y_n} \right)^{2/3} (3.75 \times 10^{-2})^{1/2}$$

$$\Rightarrow y_n = 2.296 \text{ m}$$

(ii) $y_n < y_c \Rightarrow$ reach 1 has steep slope

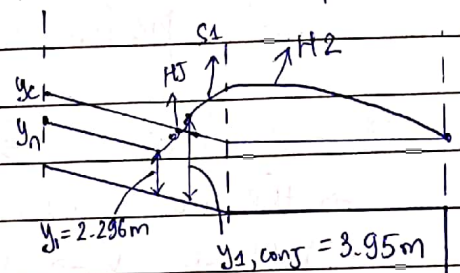


(iii) Assume the hydraulic jump will form on the upstream slope.

$$y_1 = y_n = 2.296 \text{ m}$$

$$V_1 = \frac{q}{y_1} = \frac{16.67}{2.296} = 7.260 \text{ m/s}$$

$$Fr_1^2 = \frac{V_1^2}{g y_1} = \frac{7.260^2}{9.81 \times 2.296} = 2.340$$



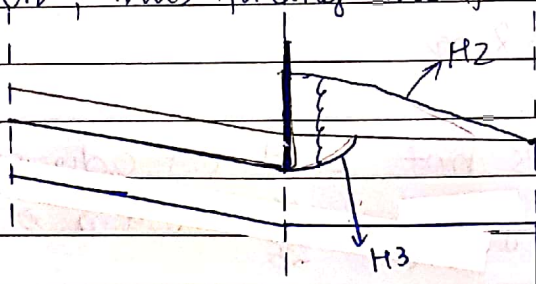
By SHJ equation: $y_{1,conj} = \frac{1}{2} y_1 (\sqrt{1 + 8 Fr_1^2} - 1)$

$$= \frac{1}{2} \times 2.296 (\sqrt{1 + 2.34 \times 8} - 1)$$

$$= 3.95 \text{ m}$$

Since the limits of $S1'$ profile are $y_c = 3.048$ and $y_p = 5.5$ m, $y_{1,conj}$ can be found on the $S1$ profile
 \therefore HJ will form on upstream slope

(iv) Put a sluice gate at the intersection of the 2 slopes, with the opening $= y_n = 2.296$ m. The sluice gate will prevent the $S1$ profile formation, thus forcing the flow to jump on the downstream slope, where $y_{1,conj}$ can be found on $H2$ profile



4.

(a)

$$(i) \frac{dy}{dx} = \frac{S_0 - S_f}{1 - Fr^2}$$

For M2 profile: $y_c < y < y_n$

- Since $y > y_c$, $Fr < 1 \Rightarrow 1 - Fr^2 > 0$

- Since $y < y_n$, $S_f > S_0 \Rightarrow S_0 - S_f < 0$

Hence $\frac{dy}{dx} = \frac{-ve}{+ve} = -ve$

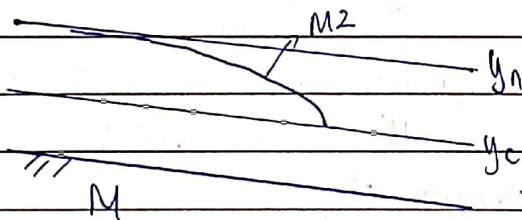
\therefore M2 profile is decreasing in the downstream direction

(ii) - When $y \rightarrow y_c$, $Fr \rightarrow 1 \Rightarrow \frac{dy}{dx} \rightarrow \infty$

\Rightarrow the flow cuts y_c -line at right angle dx

- When $y \rightarrow y_n$, $S_f \rightarrow 0 \Rightarrow \frac{dy}{dx} \rightarrow 0$

\Rightarrow the flow asymptotes to y_n line



(b)

$$(i) q = \frac{Q}{b} = \frac{400}{40} = 10 \text{ m}^2/\text{s}$$

$$y_c = \sqrt[3]{\frac{q^2}{g}} = \sqrt[3]{\frac{10^2}{9.81}} = 2.168 \text{ m}$$

normal flow depth does not exist on adverse slope

Flow depth at entrance: $y = \frac{q}{\sqrt{25}} = 0.4 \text{ m} < y_c \Rightarrow$ A3 profile



(ii)

$$y_1 = y_c = 2.168 \text{ m}$$

$$V_1 = \frac{Q}{y_1} = 4.613 \text{ m} \rightarrow Fr_1 = \frac{V_1^2}{gy_1} = 1$$

For the hydraulic jump to form on channel 1, the A2 profile must be formed on channel 1 \rightarrow the flow depth at the interface of 2 channels

Assume the channel 2 to be long must be larger than y_c

Hence channel 2 has steep slope so that the flow depth in channel 1 is A2 profile.

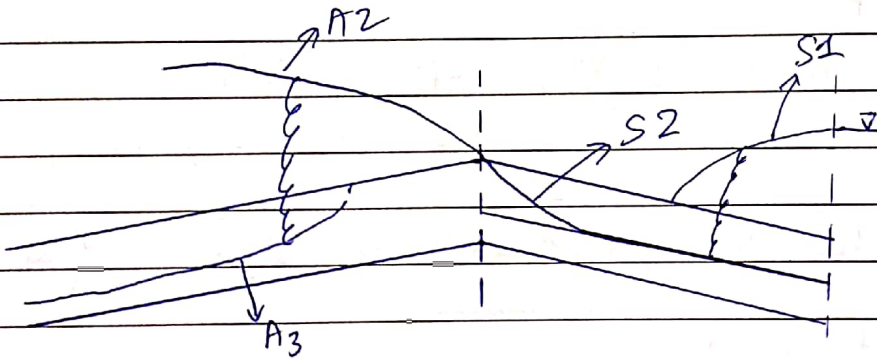
$$\Rightarrow y_n < 2.168 \text{ m}$$

$$\frac{Q}{400} = \frac{1}{n} A R_h^{2/3} S_0^{1/2} = \frac{1}{0.025} (40 y_n) \left(\frac{40 y_n}{40 + 2 y_n} \right)^{2/3} S_0^{1/2}$$

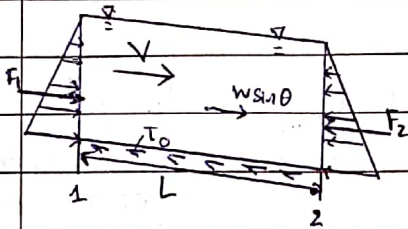
$$\text{If } y_n = 2.168 \text{ m} \Rightarrow S_0 = 5.436 \times 10^{-3}$$

$$\therefore S_0 > 5.436 \times 10^{-3}$$

(iii)



1(c) Given: $\tau_0 = \frac{\rho f V^2}{8}$; $V = \frac{1}{n} R_h^{2/3} S_0^{1/2}$



Take control volume between section 1 & section 2
in an open channel flow with bed slope $S_0 = \tan \theta$
For small θ : $\sin \theta \approx \tan \theta \Rightarrow S_0 \approx S$

Momentum equation gives: $F_{net} = \dot{M}_2 - \dot{M}_1$

$\Rightarrow F_1 - F_2 + W \sin \theta - \tau_0 \text{ opposing shear force} = \rho Q (V_2 - V_1)$

Since $V_1 = V_2 = V$ and $F_1 = F_2$ (hydrostatically distributed)

$\Rightarrow W \text{ opposing shear force} = W \sin \theta$

$\Rightarrow \tau_0 PL = \rho g AL \sin \theta$ (1)

Substitute $\sin \theta \approx \tan \theta = hf/L = S \approx S_0$ into (1):

$\tau_0 PL = \rho g AL S_0$

$\Rightarrow \tau_0 = \rho g \frac{A}{P} S_0 = \rho g R_h S_0$

Equating with $\tau_0 = \frac{\rho f V^2}{8}$: $\frac{\rho f V^2}{8} = \rho g R_h S_0$
 $\Rightarrow V = \sqrt{\frac{8g R_h S_0}{f}}$

Equating with Manning's equation $\Rightarrow \sqrt{\frac{8g R_h S_0}{f}} = \frac{1}{n} R_h^{2/3} S_0^{1/2}$

$\Rightarrow n = \sqrt{\frac{f}{8g}} R_h^{1/6}$