

Lim Kai Jian

CV2013 Engineering Geology & Soil Mechanics Semester 1 2020-2021

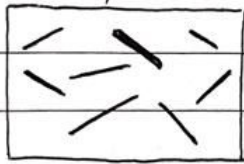
1(a) Jurong Formation Sedimentary Rocks, Bukit Timah Granite, Kallang Formation

(b) Cleavage refers to the tendency of a crystal to split along planar surfaces
Fracture is the tendency of a crystal to break along irregular surfaces other than cleavage planes

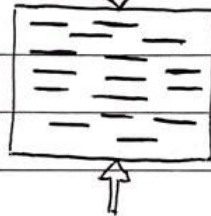
(c) Four Surface Processes: Weathering, Erosion, Transportation, Deposition
Two Surface Processes: Burial, Diagenesis

(d) Foliation is the most prominent textural feature of regionally metamorphosed rocks. It refers to a set of flat or wavy parallel cleavage planes produced by deformation of igneous and sedimentary rocks under directed pressure

Randomly Oriented Minerals



Direction of maximum stress



Preferentially Oriented Minerals

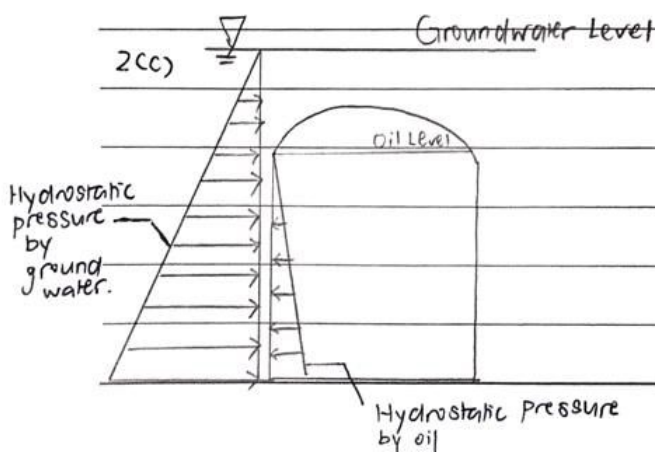
Other criteria: Metamorphic Grade, Grain size, Banding and Mineralogy

~~2(a)~~ ~~2(b)~~

2(b) In compression, the stiffness gradually increases with applied stress or displacement, reaching a limit associated w/ the strength of the intact rock.

In tension, no tensile stress can be sustained and hence the displacement increases.

In shear, the stress displacement curve is rather like the complete stress-strain curve for compression of intact rock, except that all failure is localised along the discontinuity.



Containment principle: The hydrostatic pressure at cavern depth is sufficient to counterbalance the cavern pressure and confine the stored product in the underground storage cavern. As a result of the groundwater flow into the cavern, leakage outwards is prevented.



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Q2.

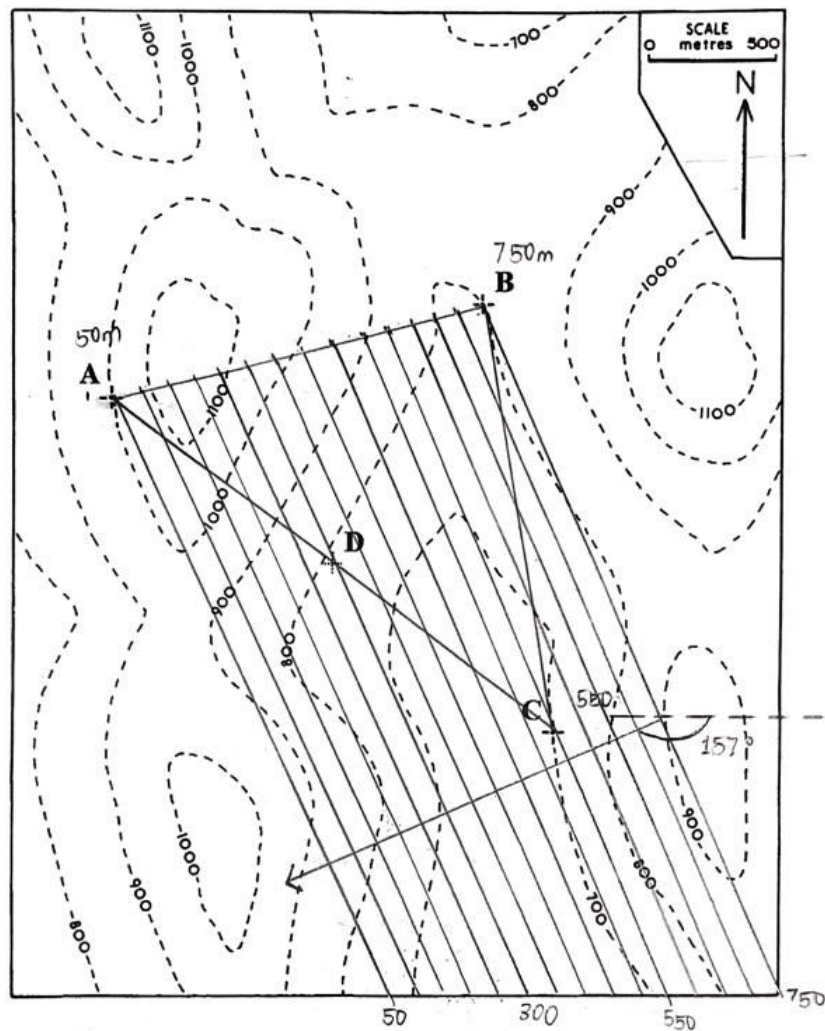
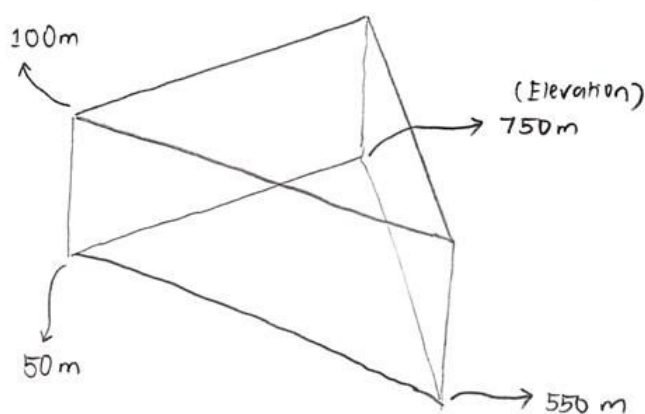


Figure Q2

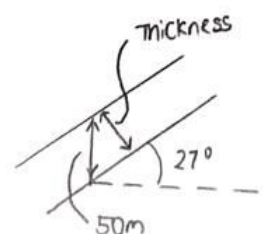


2 (a) (i) Dip Direction = $157^\circ + 90^\circ = 247^\circ //$
 Dip Angle = $\tan^{-1} \frac{\text{Rise}}{\text{Run}} = \tan^{-1} \frac{700}{1389} = 26.7^\circ \approx 27^\circ //$

(ii) Depth = $790\text{m} - 300\text{m} = 490\text{m} //$

(iii) Thickness = $\sin(90^\circ - 27^\circ) \times 50$
 $= 44.6\text{m} //$

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Let the total head btwn Soil A & Soil B be X

$$3. (a) \quad q_A = q_B$$

$$k_A \left(\frac{3.5 - X}{1} \right) A = k_B \left(\frac{X - 0}{2} \right) A$$

$$9.5 k_B (3.5 - X) = k_B \left(\frac{X}{2} \right)$$

$$33.25 - 9.5X = 0.5X$$

$$X = \frac{33.25}{10} = 3.325 \text{ m} //$$

$$(b) \quad i_A = \frac{3.5 - 3.325}{1} = 0.175 //$$

$$i_B = \frac{3.325 - 0}{2} = 1.6625 //$$

$$(c) \quad \text{Total Head @ } M_+ = 1.6625 \text{ m} = h$$

$$\Rightarrow h = 1.6625 \text{ m} //$$

$$(d) \quad q_A = k_A i_A A = 9.5 (1.0 \times 10^{-4}) (0.175) = 0.00016625 \text{ m}^3/\text{s}$$

$$\text{Time needed in hours} = \frac{1}{0.00016625} \times \frac{1}{3600} = 1.67 \text{ hours} //$$

$$(e) \quad \sigma' = \sigma - u$$

$$= (0.5 \times 10 + 1 \times 18 + \cancel{1 \times 20}) - \overbrace{(0.6625 \times 10)}^{1.6625 - 1}$$

$$= 36.375 \text{ kPa} //$$

$$4(a)(i) \quad \text{Double Drainage} \Rightarrow d = \frac{H}{2} = \frac{2.5/100}{2} = 0.0125$$

$$T_v = -0.933 \log(1 - U) - 0.085 = 0.848$$

$$T_v = \frac{C_v t}{d^2} \Rightarrow C_v = \frac{T_v \times d^2}{t}$$
$$= \frac{0.848 \times 0.0125^2}{60}$$
$$= 2.2083 \times 10^{-6}$$

$$(ii) \quad \frac{z}{d} = 1, \quad T_v = 0.848, \quad U_z \approx 0.84 \quad (\text{From Isochrone chart})$$

$$U_z = 1 - \frac{u_e}{u_i}$$

$$0.84 = 1 - \frac{u_e}{\Delta q}$$

$$u_e = (0.84 - 1)(-150) = 24 \text{ kPa} //$$



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4-(b) (i) Before Sandfill placement:

$$\text{Total vertical stress} = 2 \times 16 + 2 \times 10 = 52 \text{ kPa} //$$

$$\text{Total porewater pressure} = 4 \times 10 = 40 \text{ kPa} //$$

$$\text{Excess pwp} = 0 \text{ kPa} // \text{ (Hydrostatic)}$$

$$\text{Effective vertical stress} = 52 - 40 = 12 \text{ kPa} //$$

After Sandfill placement:

$$\text{Total vertical stress} = 2 \times 16 + 2 \times 20 + 3 \times 18 = 126 \text{ kPa} //$$

$$\text{Total porewater pressure} = 40 + 74 = 114 \text{ kPa} //$$

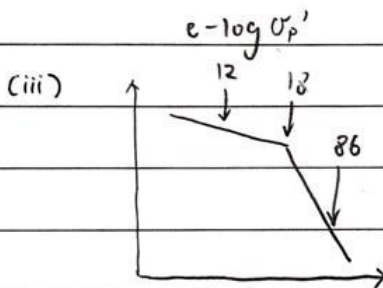
$$\text{Excess porewater pressure} = \Delta q = 2 \times 20 + 3 \times 18 - 2 \times 10 = 74 \text{ kPa} //$$

$$\text{Effective vertical stress} = 126 - 114 = 12 \text{ kPa} //$$

(ii) When clay has fully consolidated,

$$u_e = 0 \text{ kPa} \text{ (Back to hydrostatic condition)}$$

$$\text{Effective vertical stress} = ~~52~~ 12 + 74 = 86 \text{ kPa} //$$



$$\text{OCR} = 1.5 = \frac{\sigma_p'}{\sigma_{vo}'} \Rightarrow \sigma_p' = 1.5 \times 12 = 18 \text{ kPa}$$

$$S_c = \frac{4}{1+1.5} \times \left(0.05 \log \left(\frac{18}{12} \right) + 0.5 \log \left(\frac{86}{18} \right) \right) \\ = 0.557 \text{ m} //$$

(iv) Water level = 8 m

$$\text{porewater pressure} = 8 \times 10 = 80 \text{ kPa} , u_e = 80 - 40 = 40 \text{ kPa}$$

$$U_z = 1 - \frac{u_e}{u_i} = 1 - \frac{40}{74} = 0.459 \approx 0.46$$

$$\frac{z}{d} = 1$$

\Rightarrow From Isochrone chart, $T_v \approx 0.35$

$$T_v = \frac{C_v t}{d^2}$$

$$0.35 \times \left(\frac{4}{2} \right)^2 \times \frac{1}{1.25} = 1.12 \text{ years} //$$



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