



- 1 a. Viscosity indicates the stiffness and quality of the Binder, higher the viscosity, higher the quality.
In Singapore, the average temperature is around 30°C
 \rightarrow Binder A more viscous \rightarrow better.
- b. - Rough \rightarrow provide skid resistance \rightarrow no slip.
- Angular \rightarrow better packing \rightarrow prevent aggregate from flying apart when exposed to high impulse.
- Well graded ($\overset{\text{enough}}{\text{pores}}$) \rightarrow Singapore has wet weather \rightarrow prevent fog

$$\text{c. } \text{VTM} = 100 \times \frac{V_A}{V}$$

$$D = \frac{M}{V_{MM}}$$

$$2.480 = \frac{1250}{V_{MM}}$$

$$V_{MM} = 504.03 \text{ cm}^3$$

$$V = \frac{W_{\text{air}} - W_{\text{water}}}{P_{\text{water}}} \\ = \frac{1250 - 734}{1} = 516 \text{ cm}^3$$

$$V_A = V - V_{MM} = 516 - 504.03 = 11.97 \text{ cm}^3$$

$$\text{VTM} = 100 \times \frac{V_A}{V} = \frac{100 \times 11.97}{516} = 2.32\%$$

VTM for proper design is 3% - 5%. Hence, not suitable.

2 a. Above 727° \rightarrow Austenite + cementite \rightarrow 6.7% C

$$6.7\% \times \text{Cementite} + 0.8\% \times \text{Austenite} = 1.2\%$$

$$\text{Cementite} + \text{Austenite} = 1.$$

$$6.7\% \text{ Cementite} + 0.8\% (1 - \text{Cementite}) = 1.2\%$$

$$\text{Cementite} = 6.7\%$$

$$\text{Austenite} = 93.3\%$$

Below 727° \rightarrow Cementite + Pearlite \rightarrow Ferrite.

\rightarrow Austenite become Pearlite \rightarrow same carbon content (0.8%) \rightarrow calculation same

$$\hookrightarrow \text{Cementite} = 6.7\%.$$

$$\text{Pearlite} = 93.3\%.$$

Find: Cementite (6.7%) + Ferrite (0.025%).

$$6.7\% \text{ Cementite} + 0.025\% (1 - \text{Cementite}) = 1.2\%$$

$$\text{Cementite} = 17.6\% \text{ Ferrite} = 82.4\%$$

along = 6.7%

$$\rightarrow \text{Cementite in pearlite} = 93.3 - 82.4$$

$$= 10.9\%$$

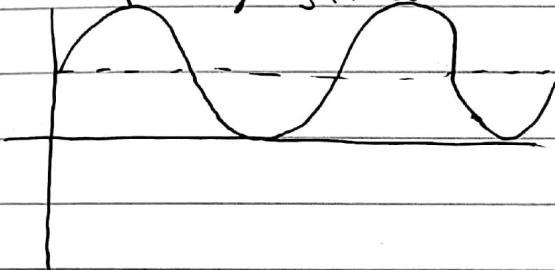


2b i

Fatigue failure is the result of processes of crack nucleation and growth or, for components which may contain cracks introduced during manufacture, growth only.

ii Trampoline \rightarrow Subjected to repeated cyclic stresses

\hookrightarrow repeating stress.



iii - increase critical crack length a_{cr} at failure

\hookrightarrow choose material with higher K_{IC}

\hookrightarrow Reduce σ_{max}

- Reduce initial flaw size a_0

\hookrightarrow Improve inspection resolution

\hookrightarrow Improve manufacturing / material quality control

- Lower stress range $\Delta\sigma$

C. Similarities \rightarrow type of wood

\rightarrow Mechanical properties are not as limited by knots.

\rightarrow Dimensionally stable.

Differences \rightarrow Plywood's grain at right angles to each other, glulam parallel.

\rightarrow glulam can be curved, plywood cannot.

\rightarrow glulam more expensive \rightarrow need glue.

D. Similarities \rightarrow increase strength, ductility, toughness

\rightarrow using fibre

\rightarrow No yielding

Differences \rightarrow Polymer matrix Vs cementitious matrix.

\rightarrow in polymer \rightarrow Fibre carries load, cementitious \rightarrow load & ductility

\rightarrow in polymer, fibre content is high, in cementitious,

Fibre content is low.



- 3 a.i Surfaces, such as roads and sidewalks, constructed from materials like concrete carry polluted storm water to storm drains, rather than allowing the water to percolate through soil.
- ii flooding & water pollution.
- iii concrete \rightarrow low permeability \rightarrow water cannot penetrate surface runoff!
- b.i Each aspect in a composite which has its own function.
- ii - Discrete phase \Rightarrow Aggregate \rightarrow Economical filler
 \rightarrow responsible for ductility.
 \rightarrow Provide dimensional stability.
- Continuous phase \rightarrow cement paste \rightarrow Binds the aggregate particles.
 \rightarrow provide strength & stiffness to concrete.
- Interfacial transition zone \rightarrow weakest link of the chain.

C. Cement A:

$$\begin{aligned} C_3S &= 4.07C - 7.60S - 6.72A - 1.43F - 2.85SO_3 \\ &= 42.56\% \end{aligned}$$

$$C_2S = 2.87S - 0.75C_3S = 30.84\% \quad \rightarrow \text{more like type 4.}$$

$$C_3A = 2.65A - 1.69F = 11.51\% \quad (\text{lower } C_3S \text{ & } C_3A \text{ content})$$

$$C_4AF = 3.04F = 7.8\%$$

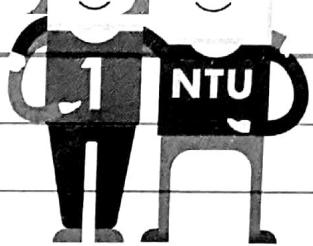
Cement B:

$$C_3S = 52.5\%$$

$$C_2S = 20.97\%$$

$$C_3A = 11.96\%$$

$$C_4AF = 7.84\%$$



3d i

$$BSG_{SSD} = \frac{W_{SSD}}{W_{PyCn,W} + W_{SSD} - W_{PyCn,Wts}}$$

$$= \frac{1000}{1390 + 1000 - 2000} = 2.564 \text{ gr/cm}^3$$

ii

$$BSG_{SSD} = \frac{W_{SSD}}{W_{PyCn,Wt} + W_{SSD} - W_{PyCn,Wts}}$$

$$2.564 = \frac{W_{SSD}}{1390 + W_{SSD} - 1691}$$

$$W_{SSD} = 493.46 \text{ gr.}$$

$$A = \frac{W_{SSD} - W_{Op}}{W_{Op}} \times 100 = \frac{493.46 - 490}{490} \times 100\% = 0.71\%$$

$$M = \frac{SCO - 490}{490} \times 100 = 2.04\%$$

$$\text{Free moisture} = M - A = 1.33\%.$$

4a.

Air-entraining uses Admixtures to produce tiny, dispersed bubble into the concrete. Air bubble become the lubricant in concrete to improve workability until it is "locked" into cement paste during hardening.

b. ① R → finer aggregate → higher surface area → less water for workability.

② I → act as lubricant because of round shape.

③ N → ice is only used for controlling the cooling of concrete. ^{not suitable}

④ I → Rounded → less surface area → more water for workability.

c. lower the loading rate, lower the strength of concrete, it is because the crack needs time to propagate until failure, if the high rate, the crack doesn't have enough time to propagate resulting in higher strength.



4d i

A shrinkage that cannot be reversed even after the concrete re-stored in water.

ii

Because ~~the~~ cracks have been formed because of the shrinkage.

(e.)
not sure

$$R = \frac{PL}{bd^2} = \frac{4000 \times 400 \times 10^{-3}}{500 \times 10^{-3} \times (100 \times 10^{-3})^2} = 320000 \text{ Pa}$$