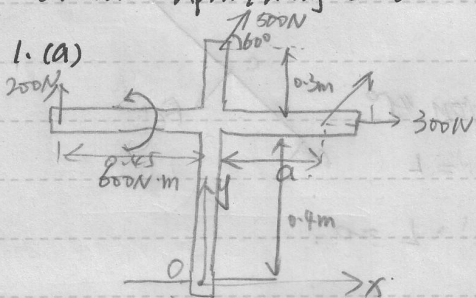


CV1011 April / May 2016

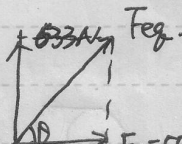
1. (a)



$$F_{eqx} = \sum F_x = 300 + 500 \cdot \cos 60^\circ = 550 \text{ N}$$

$$F_{eqy} = \sum F_y = 200 + 500 \cdot \sin 60^\circ \approx 633 \text{ N}$$

$$F_{eq} = \sqrt{F_{eqx}^2 + F_{eqy}^2} \approx \sqrt{550^2 + 633^2} \approx 839 \text{ N}$$



$$\tan \theta = \frac{633}{550} = 1.15$$

$$\theta = \tan^{-1}(1.15) \approx 49.0^\circ$$

Suppose moment anticlockwise is positive  $M \uparrow$ .

$$\uparrow \sum M_{eq} = 600 - 200 \times 0.45 - 500 \cdot \cos 60^\circ \times 0.7 - 300 \times 0.4 = 215 \text{ N}\cdot\text{m}$$

Assume the location of equivalent force is located at  $(a, 0.4)$

$$215 = 633 \times a - 550 \times 0.4$$

$$\Rightarrow a = 0.6872 \text{ m} \quad (> 0.45 \text{ m})$$

which is actually outside the frame

Assume the location is at  $(0, b)$

$$215 = -550 \times b$$

$$\Rightarrow b = -0.3909 \text{ m} \quad (< 0)$$

which is below the original point  $O$

(b) additional condition:  $L = 25 \text{ cm}$  (in exam)  $\because$  the lever in the position  $45^\circ$

$$F = k(L - L_0)$$

$$\therefore L_{AD} = \sqrt{2} \cdot L$$

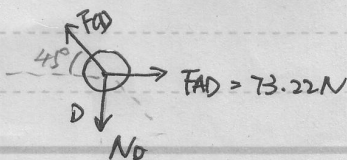
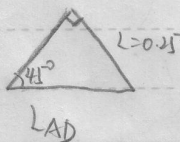
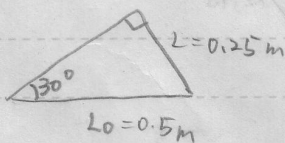
$$= 500(0.354 - 0.5)$$

$$= \sqrt{2} \times 0.25$$

$$= -73.22 \text{ N}$$

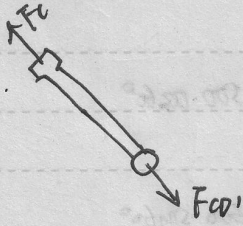
$$\approx 0.354 \text{ m}$$

negative sign means the force is compressive.



$$F_{CD} = \sqrt{2} F_{AD}$$

$$= 103.55 \text{ N}$$

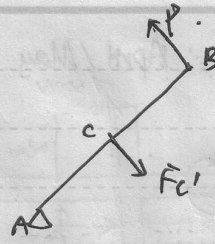


$$F_c = F_{cd}' = F_{cd} = 103.55 \text{ N}$$

$$F_{c'} = F_c = 103.55 \text{ N}$$

∴ the lever in the position  $45^\circ$

$$l_{cd} = l_{ac} = 0.25 \text{ m} = L$$

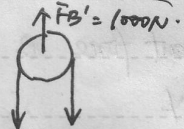
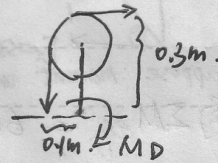
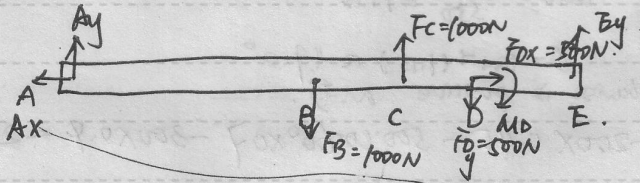


$$\sum M_A = P \cdot 2L - F_{c'} \cdot L = 0$$

$$\therefore P = \frac{1}{2} F_{c'}$$

$$P = 51.8 \text{ N}$$

2. (a)



$$M_D = 500 \times 0.3 - 500 \times 0.1 = 100 \text{ N}\cdot\text{m}$$

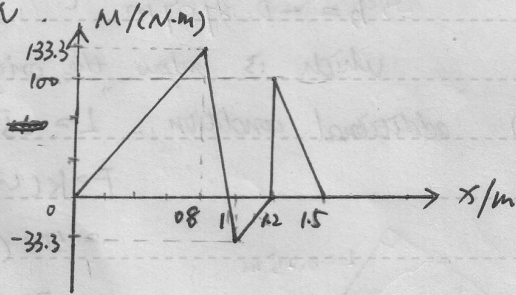
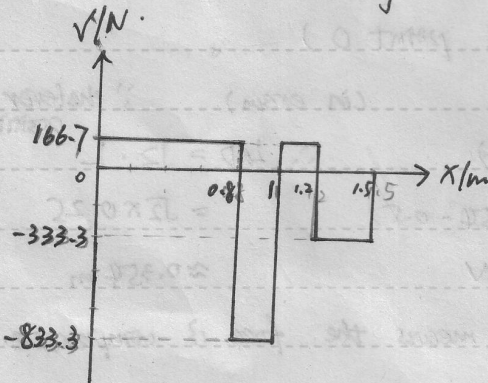
$$T = 500 \text{ N}$$

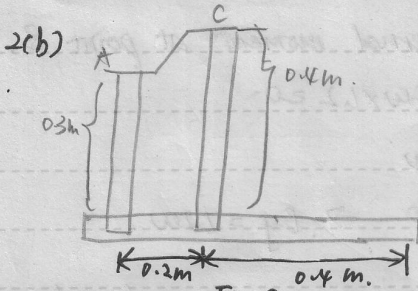
$$\sum M_A = 1000 \times 0.8 - 1000 \times 1 + (100 + (E_y)) \times 1.5 = 0$$

$$E_y = 333.3 \text{ N}$$

$$\sum F_y = A_y + 1000 - 1000 - 500 - 333.3 = 0$$

$$A_y = 166.7 \text{ N}$$



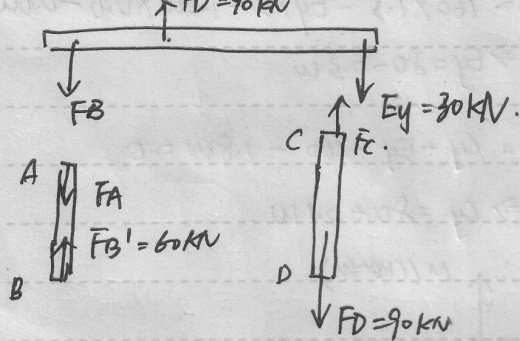


$$\sum M_B = 30 \times 0.6 - F_D \times 0.2 = 0$$

$$\Rightarrow F_D = 90 \text{ kN}$$

$$\sum F_y = 90 - F_B - 30 = 0$$

$$F_B = 60 \text{ kN}$$



$$A_{AB} = 500 \text{ mm}^2 \quad A_{CD} = 600 \text{ mm}^2$$

$$E_{st} = 700 \text{ GPa}$$

$$E_d = 70 \text{ GPa} \quad E_{st} = 200 \text{ GPa}$$

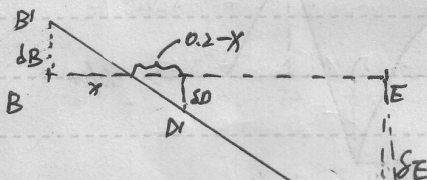
$$\delta_{AB} = \frac{P_{AB} L}{E_d A_{AB}} = \frac{60 \times 10^3 \times 0.3}{70 \times 10^9 \times 500 \times 10^{-6}}$$

$$= 5.14 \times 10^{-4} \text{ m}$$

$$= 0.514 \text{ mm} \quad (\uparrow)$$

$$\delta_{CD} = \frac{P_{CD} L}{E_{st} A_{CD}} = \frac{90 \times 10^3 \times 0.4}{200 \times 10^9 \times 600 \times 10^{-6}}$$

$$= 0.300 \text{ mm} \quad (\downarrow)$$



$$\frac{\delta_B}{\delta_D} = \frac{x}{0.2 - x} = \frac{0.514}{0.300}$$

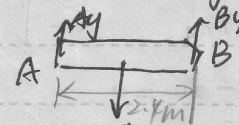
$$\Rightarrow x = 0.1263 \text{ mm}$$

$$\frac{\delta_B}{x} = \frac{\delta_E}{0.6 - x}$$

$$\delta_E = \frac{0.514 \times (0.6 - 0.1263)}{0.1263}$$

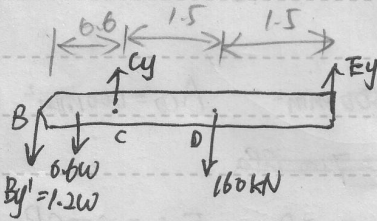
$$\delta_E = 1.928 \text{ mm} \quad (\downarrow)$$

3. (a) B is a hinge, therefore there is no internal moment at point B.

(i)   $\sum M_B = A_y \times 2.4 - 2.4W \times 1.2 = 0$

$\Rightarrow A_y = 1.2W$

$\sum F_y = A_y + B_y - 2.4W = 0 \Rightarrow B_y = 1.2W$

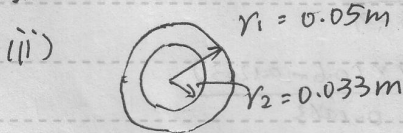
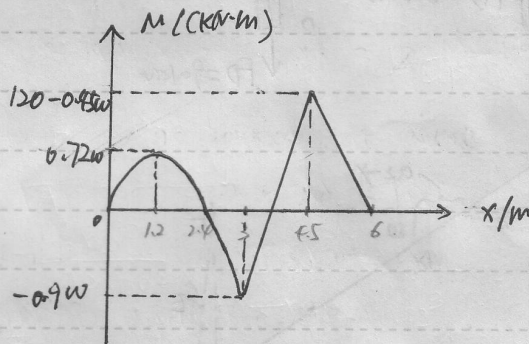
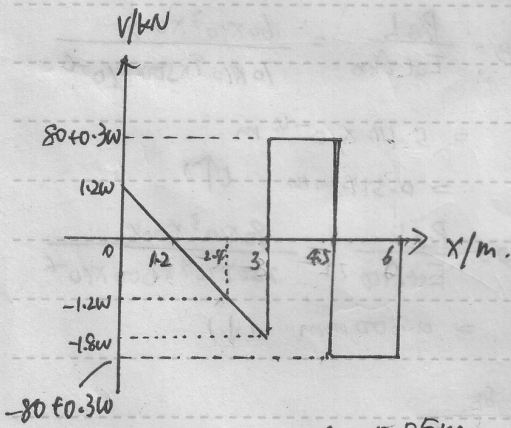


$\sum M_C = 160 \times 1.5 - E_y \times 3 - 1.2W \times 0.6 - 0.6W \times 0.3 = 0$

$\Rightarrow E_y = 80 - 0.3W$

$\sum F_y = C_y + E_y - 160 - 1.8W = 0$

$\Rightarrow C_y = 80 + 2.1W$



①  $80 + 0.3W \geq 1.8W$

$W \leq 53.33 \text{ kN/m}$

$Q = \frac{1}{2} \pi r_2^2 \times \frac{4r_2}{3\pi} - \frac{1}{2} \pi r_1^2 \times \frac{4r_1}{3\pi}$   
 $= \frac{2}{3} (0.05^3 - 0.0333^3)$   
 $= 5.937 \times 10^{-5} \text{ m}^3$

$I = \frac{VQ}{It} = \frac{(80 + 0.3W) \times 5.937 \times 10^{-5}}{3.977 \times 10^{-6} \times (0.1 - 0.0066)} \leq 60 \times 10^3 \text{ kN/m}^2$

$180 + 0.3W \leq 136.65$

$W \leq -144.5 \text{ kN/m (X)}$

$I = \frac{1}{4} \pi (r_2^4 - r_1^4)$   
 $= \frac{\pi}{4} (0.05^4 - 0.0333^4)$   
 $= 3.977 \times 10^{-6} \text{ m}^4$

②  $80 + 0.3W = 1.8W$

$W > 53.33$

$\tau = \frac{VQ}{It} = \frac{1.8W \times 5.937 \times 10^{-5}}{3.977 \times 10^{-6} \times (0.034)} \leq 60 \times 10^3 \text{ kN/m}^2$

$1.8W \times 439.07 \leq 60 \times 10^3$

$W \leq 75.91 \text{ kN/m} \#$

②  $120 - 0.45W \geq \overset{0.9}{\cancel{120}} W$   
 $W \leq \overset{88.89}{\cancel{120}}$

$$\sigma = \frac{My}{I} = \frac{(120 - 0.45W) \times 0.05}{3.977 \times 10^{-6}} \leq 120 \times 10^3 \text{ kN/m}^2$$

$$W \geq 245.5 \text{ (X)}$$

③  $W > \overset{88.89 \text{ kN/m}}{\cancel{120}}$

$$\sigma = \frac{My}{I} = \frac{0.9W \times 0.05}{3.977 \times 10^{-6}} \leq 120 \times 10^3 \text{ kN/m}^2$$

$$W \leq 10.6 \text{ (X)}$$

Therefore, the largest permissible value of W is 75.91 kN/m.

(b)

$$\begin{cases} \phi_{st} = \phi_{al} \\ T = T_{st} + T_{al} \\ T_{st} \leq 300 \text{ MPa} \end{cases}$$

$$\tau_{max\ st} = \frac{Tp}{J} = \frac{T_{st} \times 0.04}{2.749 \times 10^{-6}} \leq 300 \times 10^6$$

$$T_{st} = 20.6 \text{ kN}\cdot\text{m}$$

$$J = \frac{\pi}{32} (0.04^4 - 0.03^4) = 2.749 \times 10^{-6} \text{ m}^4$$

$$\frac{\phi_{st}}{\phi_{al}} = \frac{\frac{T_{st}}{G_{st} J_{st}}}{\frac{T_{al}}{G_{al} J_{al}}} = \frac{T_{st}}{T_{al}} \cdot \frac{G_{al} J_{al}}{G_{st} J_{st}}$$

$$J_{al} = \frac{\pi}{32} (0.03^4) = 1.272 \times 10^{-6} \text{ m}^4$$

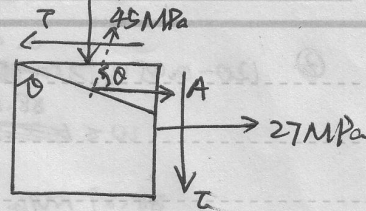
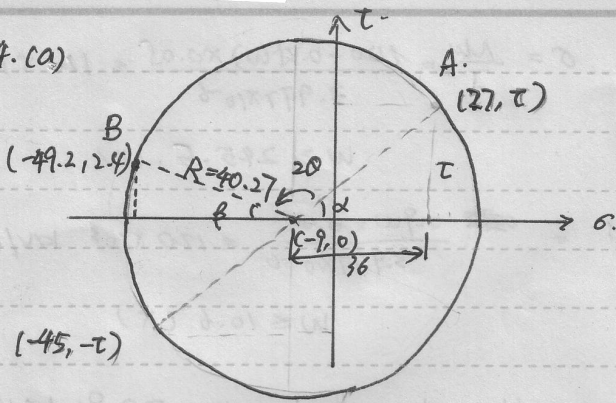
$$\Rightarrow \frac{T_{al}}{T_{st}} = \frac{G_{al} J_{al}}{G_{st} J_{st}}$$

$$= \frac{27 \times 10^9 \times 1.272 \times 10^{-6}}{77.2 \times 10^9 \times 2.749 \times 10^{-6}} = 0.1618$$

$$\begin{aligned} T &= T_{st} + T_{al} \\ &= 20.6 + 3.33 \\ &= 23.93 \text{ kN}\cdot\text{m} \end{aligned}$$

$$T_{al} = 20.6 \times 0.1618 \approx 3.33 \text{ kN}\cdot\text{m}$$

4. (a)



$$\sigma_{ave} = \frac{27 + (-45)}{2} = -9$$

$$R = \sqrt{[-49.2 - (-9)]^2 + 2.4^2}$$

$$R = 40.27$$

$$R = \sqrt{36^2 + \tau^2} = 40.27$$

$$\Rightarrow \tau = 18.05 \text{ MPa} \#$$

$$\tan \beta = \frac{2.4}{49.2} \Rightarrow \beta = 2.8^\circ$$

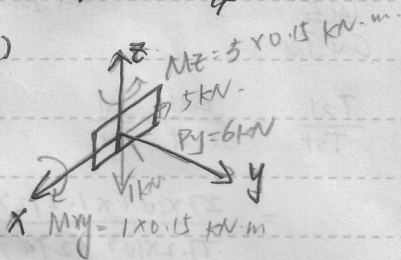
$$\theta = \frac{180 - 2.8 - 26.6}{2}$$

$$\tan \alpha = \frac{\tau}{36} = \frac{18.05}{36} \approx 0.5014$$

$$\tan^{-1}(0.5014) = 26.6^\circ$$

$$\Rightarrow \theta = 75.3^\circ \#$$

(b)

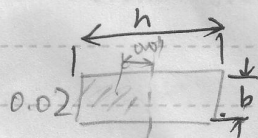


$$\sigma_x = \frac{P_y}{A} + \frac{M_x \cdot y}{I_x}$$

$$= -\frac{6 \times 10^3}{0.02 \times 0.04} + \frac{0.15 \times 10^3 \times 0.01}{\frac{1}{12} \times 0.04 \times 0.02^3}$$

$$= -7.5 + 56.26 \text{ MPa}$$

$$= 48.75 \text{ MPa}$$



$$\tau_{xy} = \frac{VQ}{It} = \frac{5 \times 10^3 \times 4 \times 10^{-6}}{1.067 \times 10^{-7} \times 0.02} = 9.372 \text{ MPa}$$

$$Q = \frac{0.01}{y} \times 0.02 \times 0.02 = 4 \times 10^{-6} \text{ m}^3$$

$$I = \frac{1}{12} \times \frac{0.02}{b} \times \frac{0.04}{b}^3 = 1.067 \times 10^{-7} \text{ m}^4$$

maximum in plane shear stress =

$$\sigma_{1,2} = \frac{\sigma_x + 0}{2} \pm \sqrt{\left(\frac{\sigma_x - 0}{2}\right)^2 + \tau_{xy}^2}$$

$$\tau_{max} = \sqrt{\left(\frac{\sigma_x}{2}\right)^2 + \tau_{xy}^2} = 26.12 \text{ MPa} \# \quad \sigma_1 = 24.375 + \sqrt{(24.375)^2 + 9.372^2}$$

maximum tensile stress,  $\sigma_1 \approx 50.49 \text{ MPa} \#$

maximum compressive stress,  $\sigma_2 = 24.375 - 26.115 = -1.74 \text{ MPa} \#$